

DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY: RWM

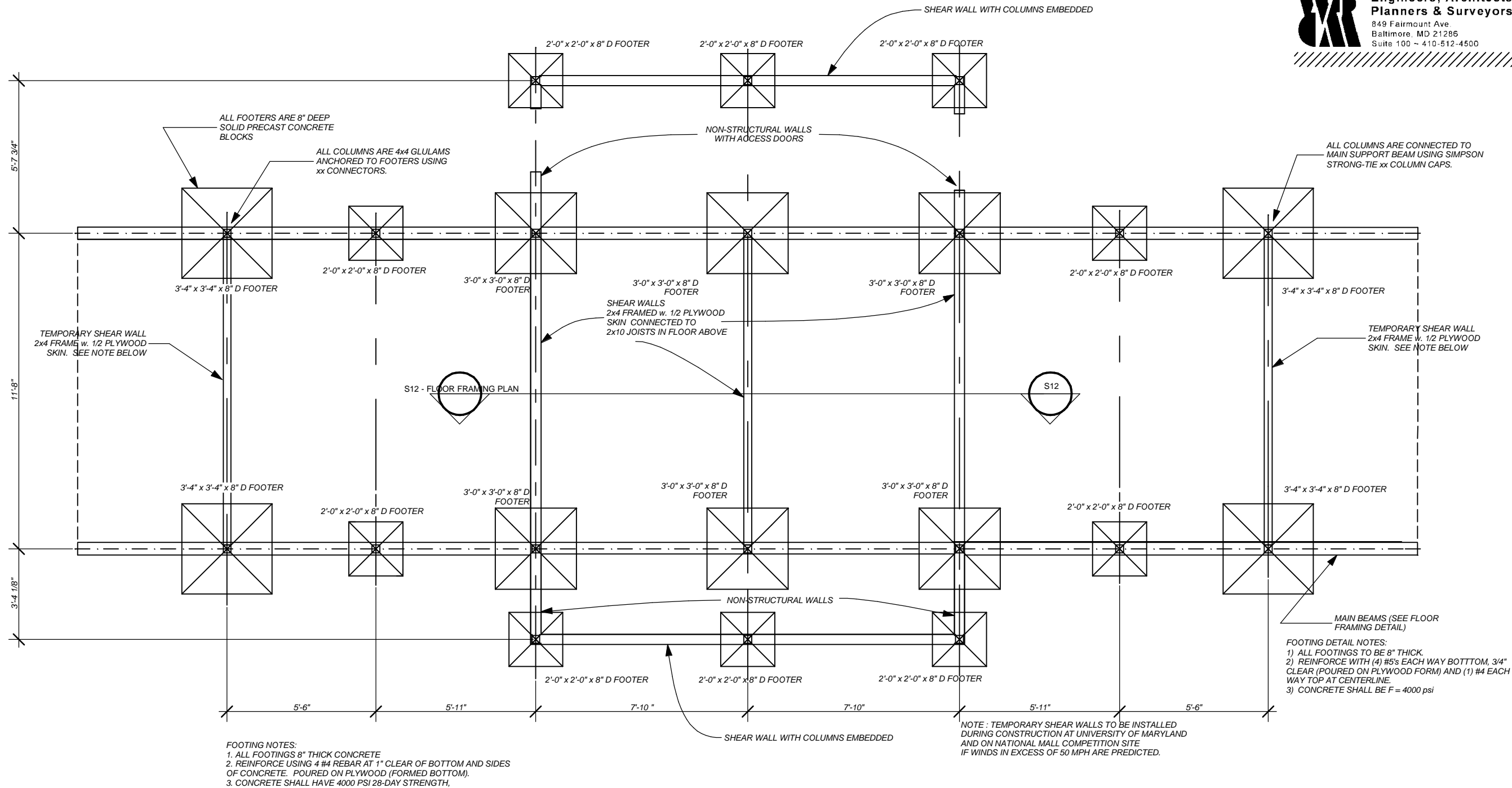
COPYRIGHT: UMD

SHEET TITLE

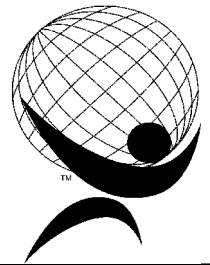
Footers Plan

S-01

SHEET 26 OF 57



1 S01 PORTABLE FOOTING PLAN
 1/4" = 1'-0"



DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY: RWM

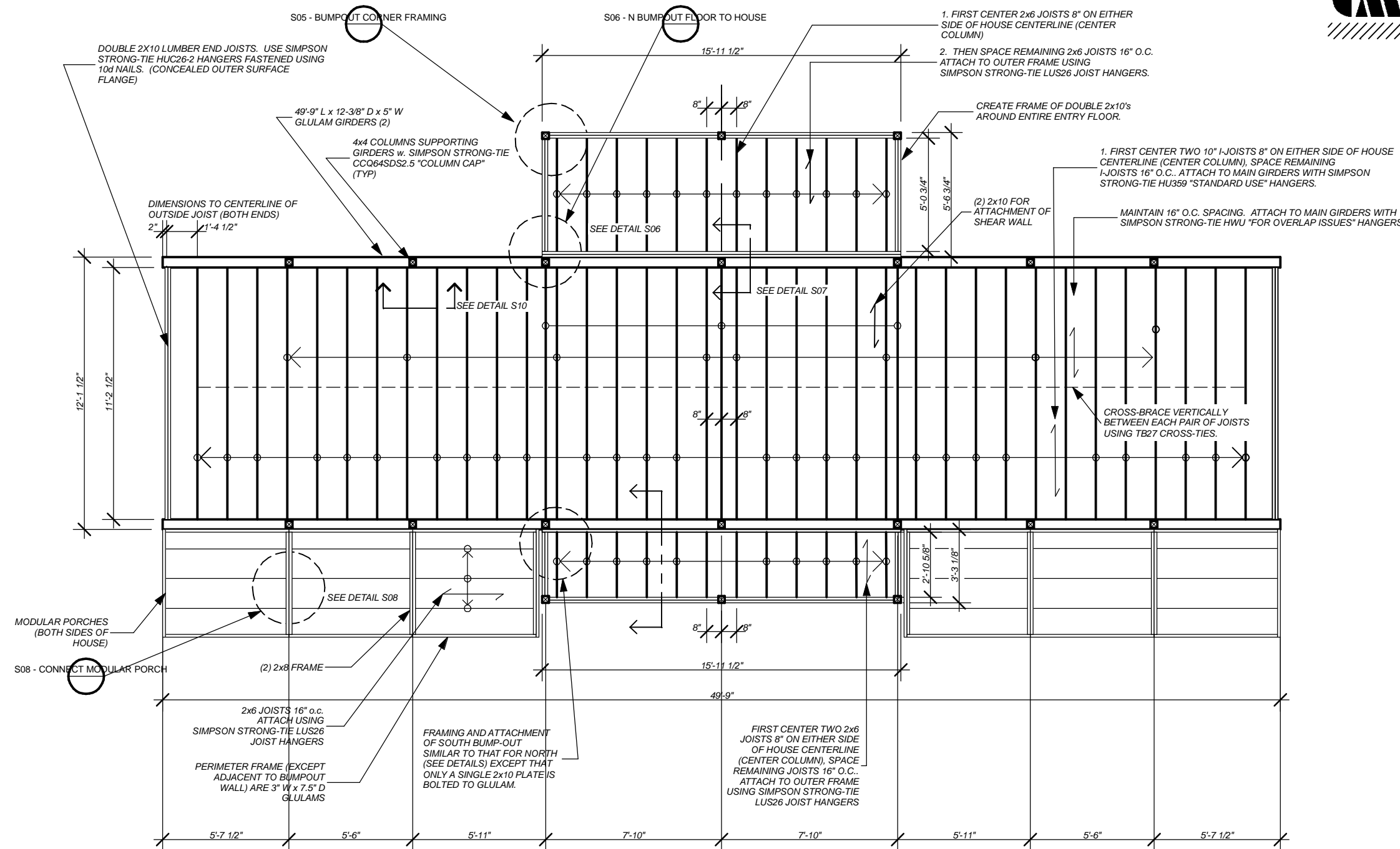
COPYRIGHT: UMD

SHEET TITLE

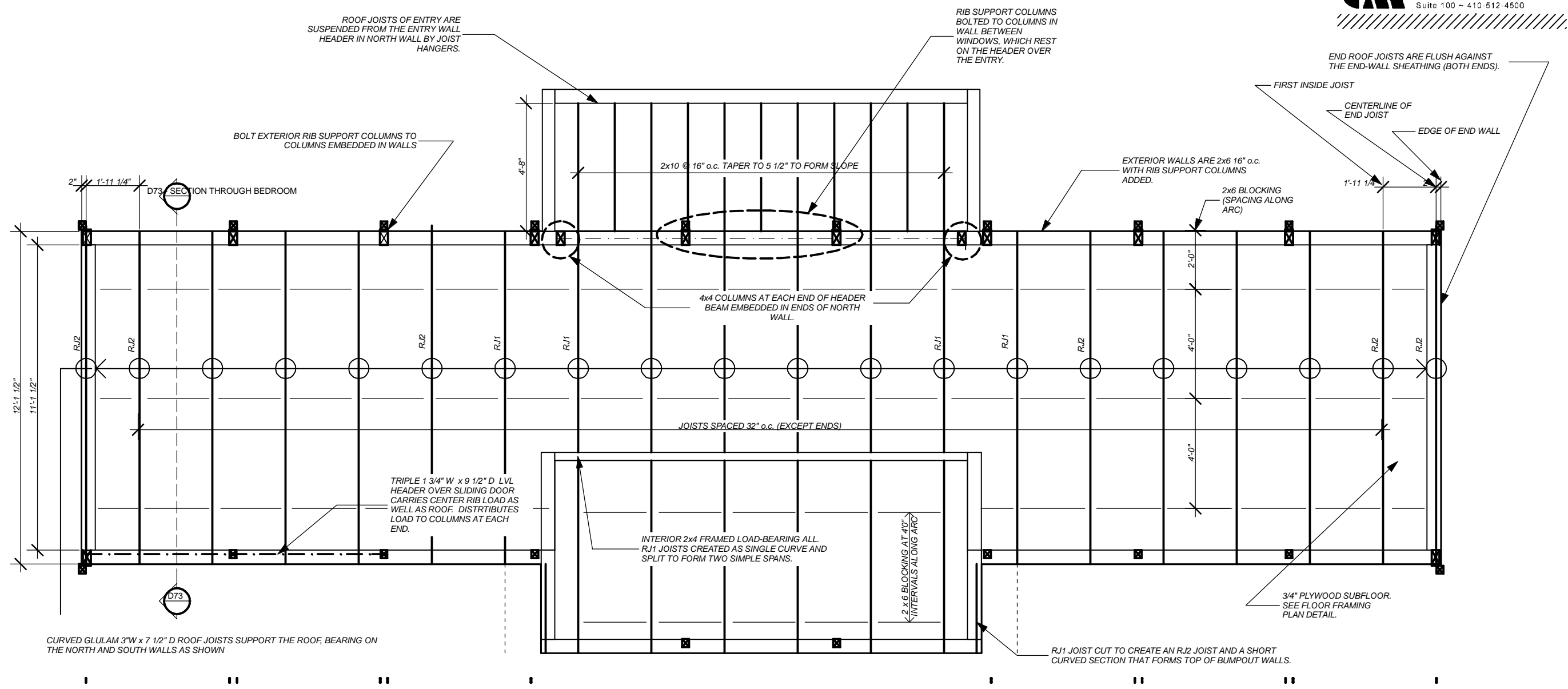
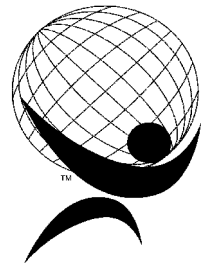
Floor Framing

S-02

SHEET 27 OF 57



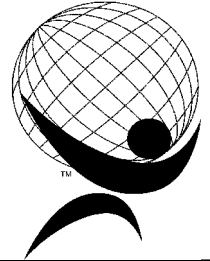
1 S02 FLOOR FRAMING
 3/16" = 1'-0"



DATE: 08/12/05
CAD NAME: 08.06.05
DESIGNED BY: UMD
DRAWN BY: UMD
CHECKED BY:RWM
COPYRIGHT: UMD

SHEET TITLE
Roof Framing

1 S03 ROOF FRAMING PLAN
 1/4" = 1'-0"

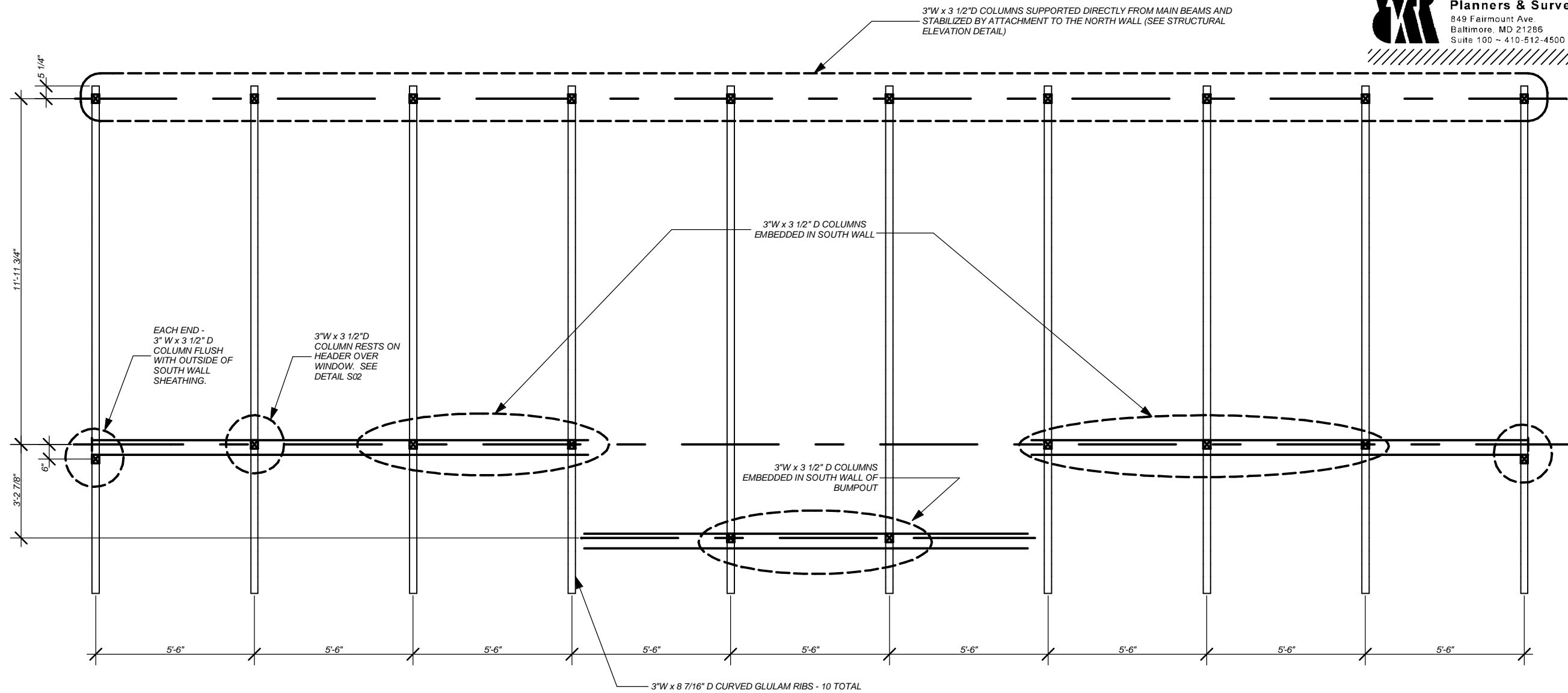


DATE: 08/12/05
 CAD NAME: 08.06.05
 DESIGNED BY: UMD
 DRAWN BY: UMD
 CHECKED BY: RWM
 COPYRIGHT: UMD

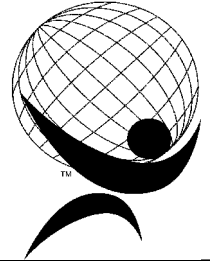
SHEET TITLE
**Rib Structure
 Plan**

S-04

SHEET 29 OF 57



1 **S04 RIB STRUCTURE PLAN**
 1/4" = 1'-0"



UNIVERSITY OF MARYLAND

University Of Maryland
2005 Solar Decathlon



DATE: 08/12/05

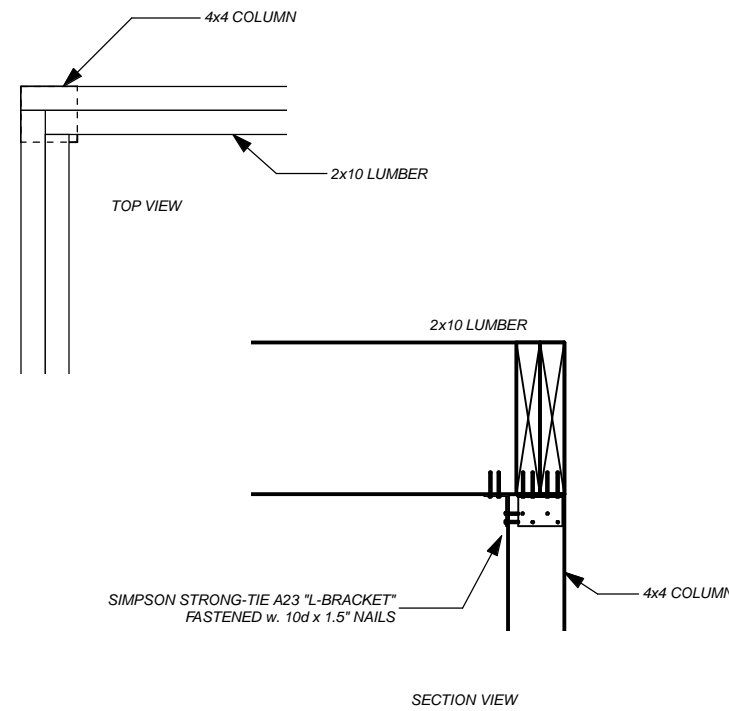
CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY:RWM

COPYRIGHT: UMD



1

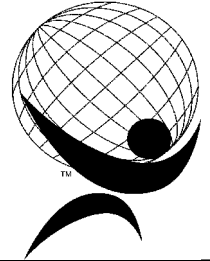
S05 BUMP-OUT CORNER FRAMING PLAN

1" = 1'-0"

SHEET TITLE
Bump-Out
Framing

S-05

SHEET 300F 57



DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY: RWM

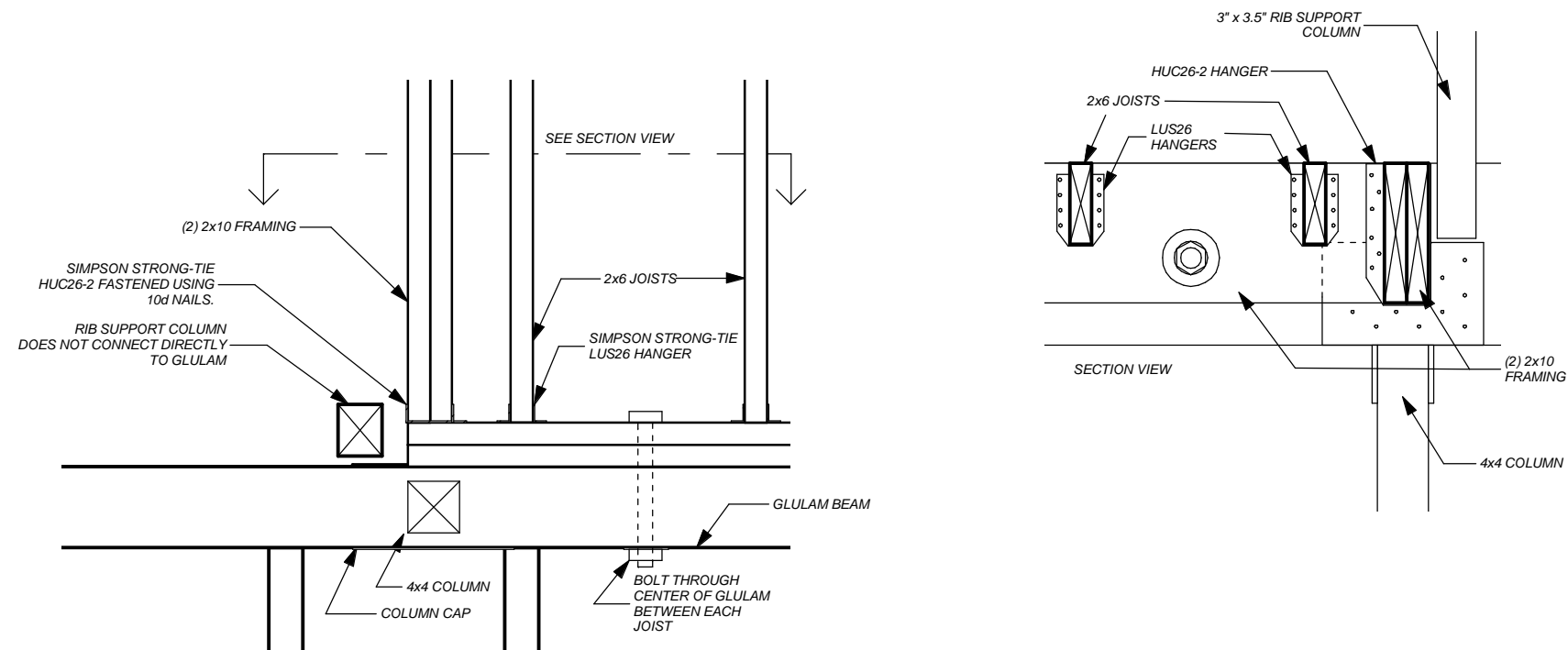
COPYRIGHT: UMD

SHEET TITLE

**Bump-Out
Details**

S-06

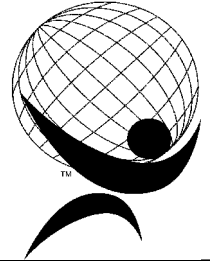
SHEET 31 OF 57



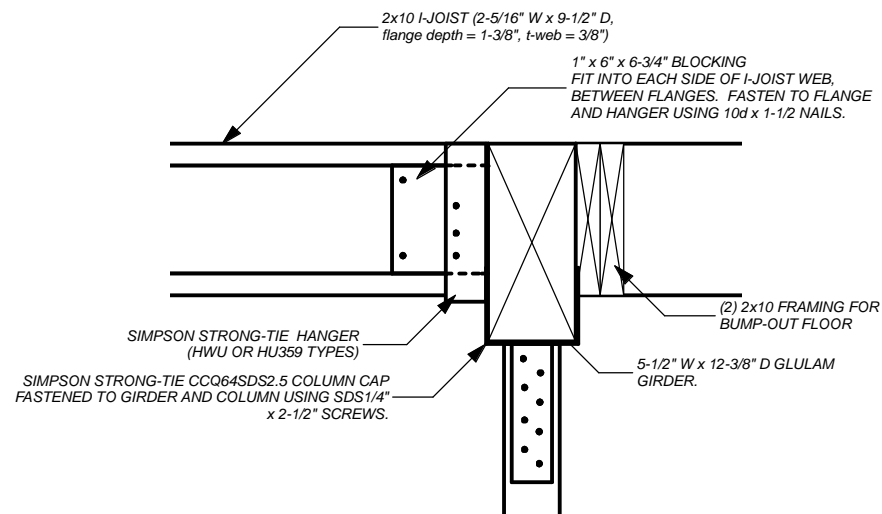
1

S06 N BUMP-OUT FLOOR TO HOUSE

1" = 1'-0"



DATE: 08/12/05
 CAD NAME: 08.06.05
 DESIGNED BY: UMD
 DRAWN BY: UMD
 CHECKED BY: RWM
 COPYRIGHT: UMD



SECTION VIEW OF I-JOIST CONNECTION TO GIRDER SHOWING REQUIRED BLOCKING FOR ATTACHMENT TO HANGER.

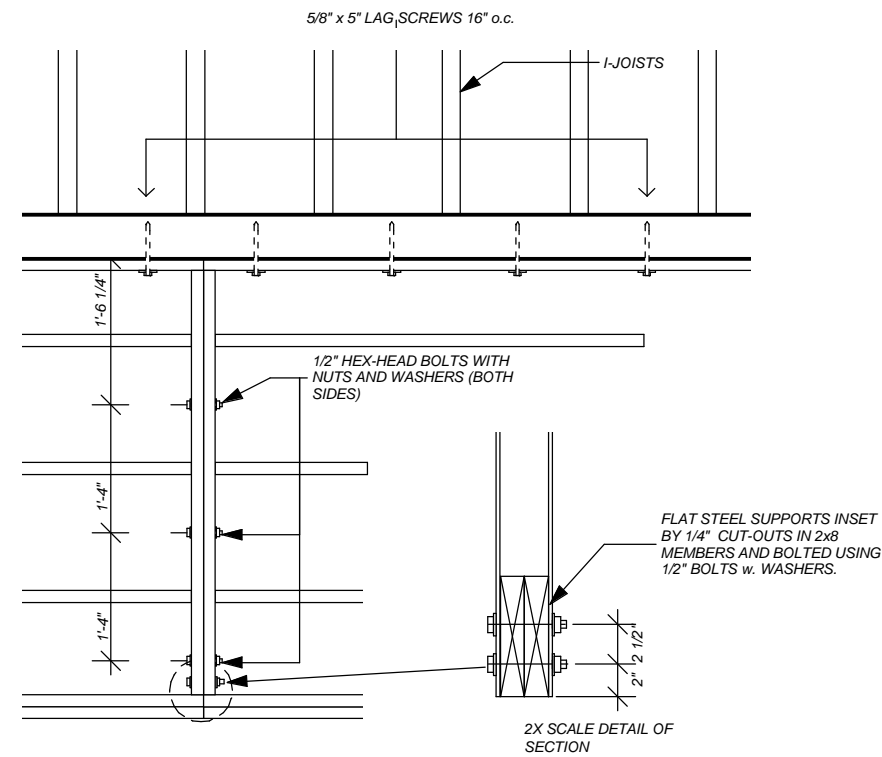
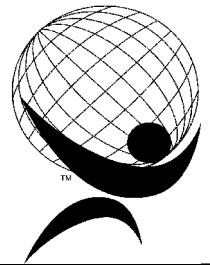
1 S07 I-JOIST CONNECT TO GIRDER
 1" = 1'-0"

SHEET TITLE

Connections

S-07

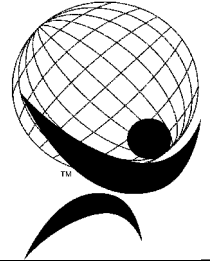
SHEET 32 OF 57



1 **S08 CONNECT MODULAR PORCH**
 1/2" = 1'-0"

SHEET TITLE
Porch
Connections

S-08



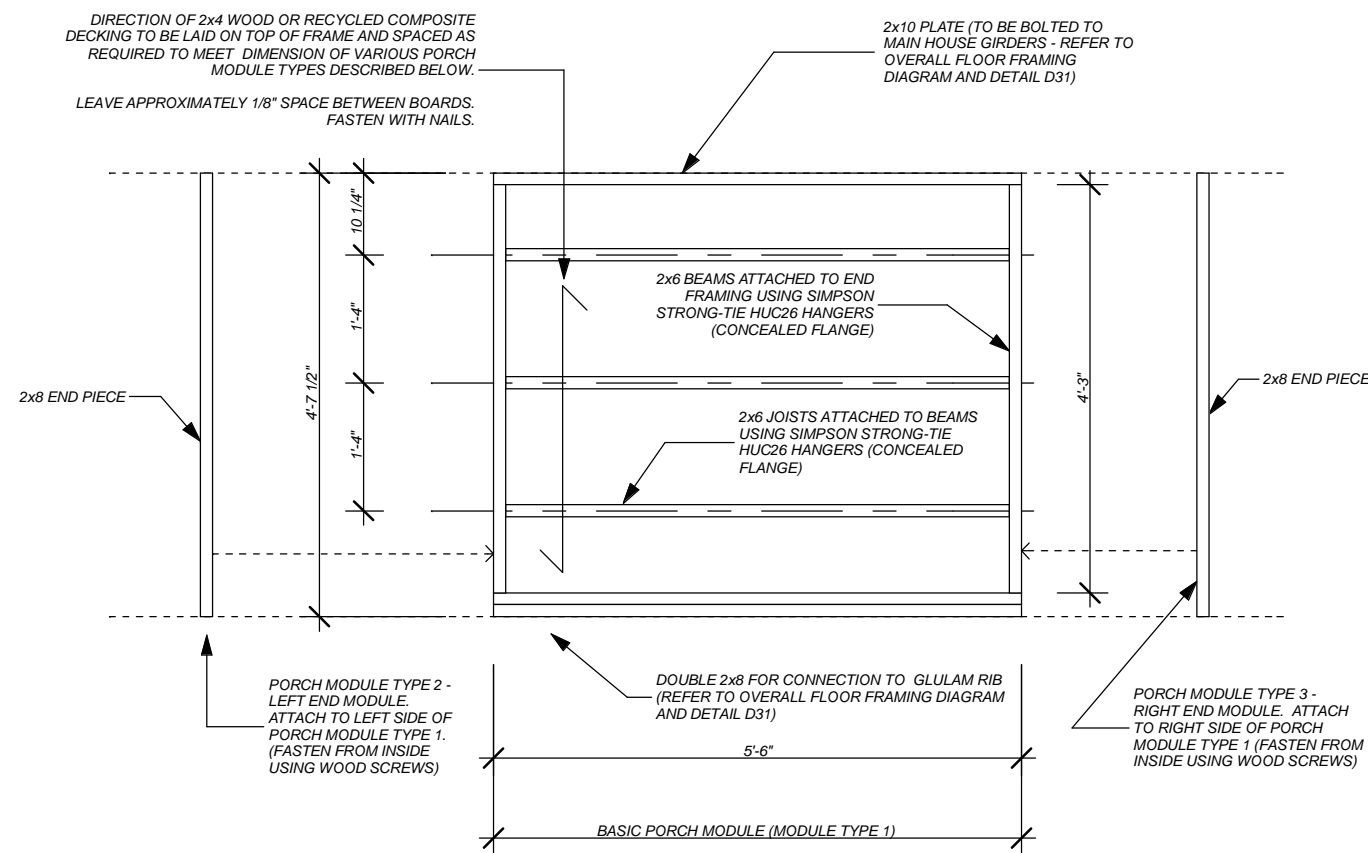
DATE: 08/12/05
 CAD NAME: 08.06.05
 DESIGNED BY: UMD
 DRAWN BY: UMD
 CHECKED BY: RWM
 COPYRIGHT: UMD

SHEET TITLE

Porch Modules

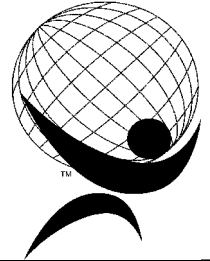
S-09

SHEET 34 OF 57



NOTE: THERE IS ONE BASIC TYPE OF PORCH MODULE AS DETAILED ABOVE (TYPE 1). TYPES 2 & 3 AS CALLED OUT IN THE OVERALL FLOOR FRAMING DIAGRAM ARE USED ONE THE LEFT AND RIGHT ENDS OF THE PORCH. THESE ARE MODIFIED BY ATTACHING AN ADDITIONAL 2x6 TO THE LEFT OR RIGHT SIDE AS DESCRIBED ABOVE.

1 S09 PORCH MODULES
 1/2" = 1'-0"



UNIVERSITY OF
MARYLAND

University Of Maryland
 2005 Solar Decathlon



DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY: RWM

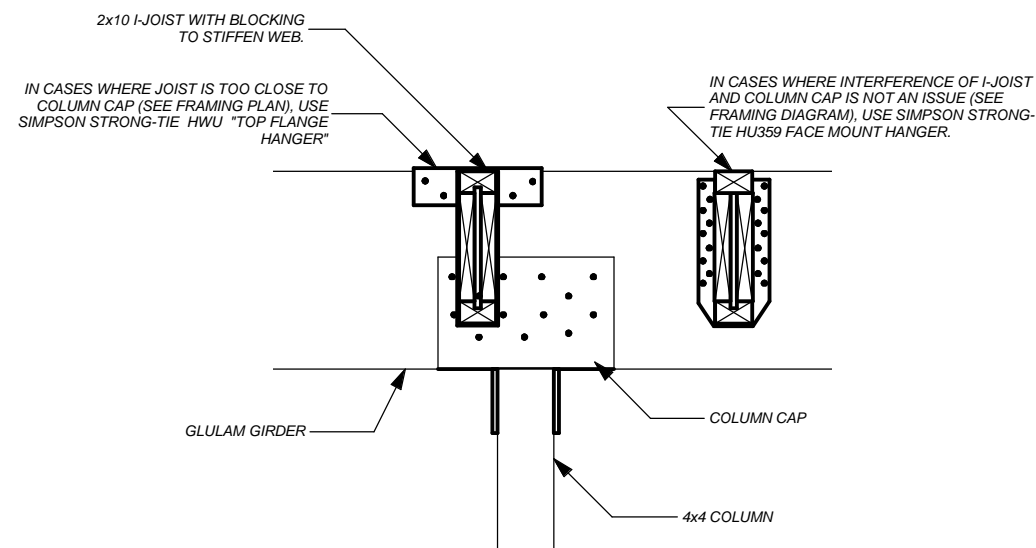
COPYRIGHT: UMD

SHEET TITLE

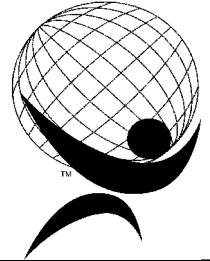
Joist Hangers

S-10

SHEET 35 OF 57



1 S10 JOIST HANGER TYPES
 1" = 1'-0"



UNIVERSITY OF
 MARYLAND

University of Maryland
 2005 Solar Decathlon



DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY: RWM

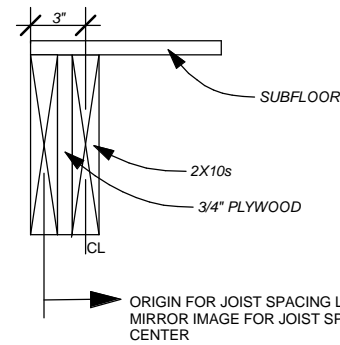
COPYRIGHT: UMD

SHEET TITLE

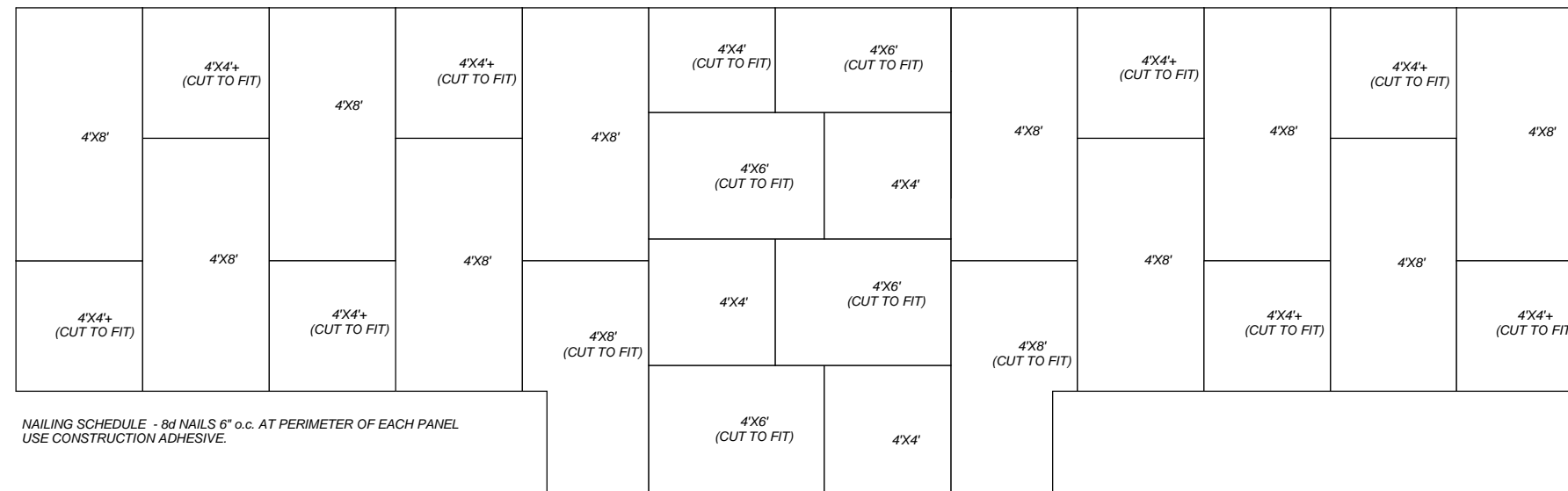
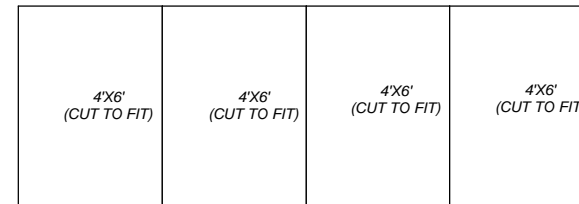
**Floor
 Diaphragm**

S-11

SHEET 36 OF 57



ORIGIN FOR JOIST SPACING LEFT OF HOUSE CENTER.
 MIRROR IMAGE FOR JOIST SPACING RIGHT OF HOUSE
 CENTER

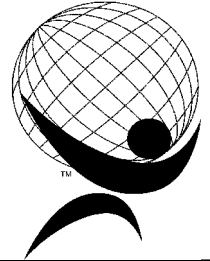


NAILING SCHEDULE - 8d NAILS 6" o.c. AT PERIMETER OF EACH PANEL
 USE CONSTRUCTION ADHESIVE.

1

S11 FLOOR DIAPHRAGM

3/16" = 1'-0"



DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY: RWM

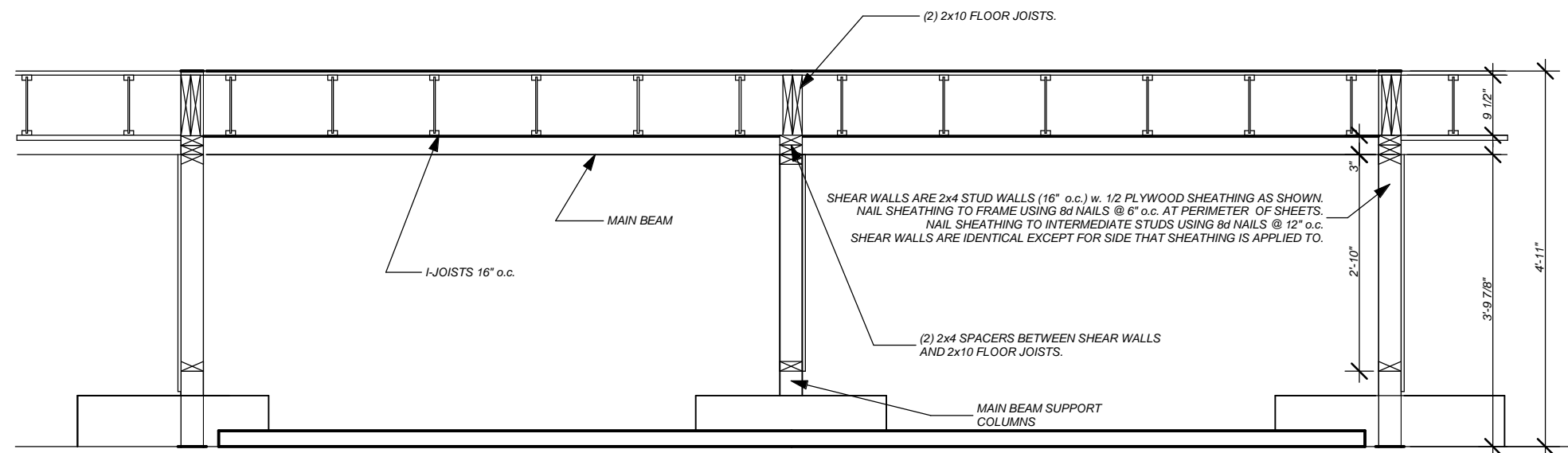
COPYRIGHT: UMD

SHEET TITLE

**Shear Wall
Section**

S-12

SHEET 37 OF 57

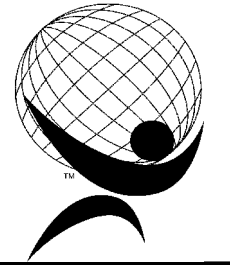


NOTE: EAST-WEST SHEAR WALLS AND TEMPORARY EMERGENCY SHEAR WALLS ARE BLOCKED AND ATTACHED TO THE FLOOR JOISTS IN THE SAME MANNER AS SHOWN HERE.

1

S12 SHEAR WALL SECTION

1/2" = 1'-0"



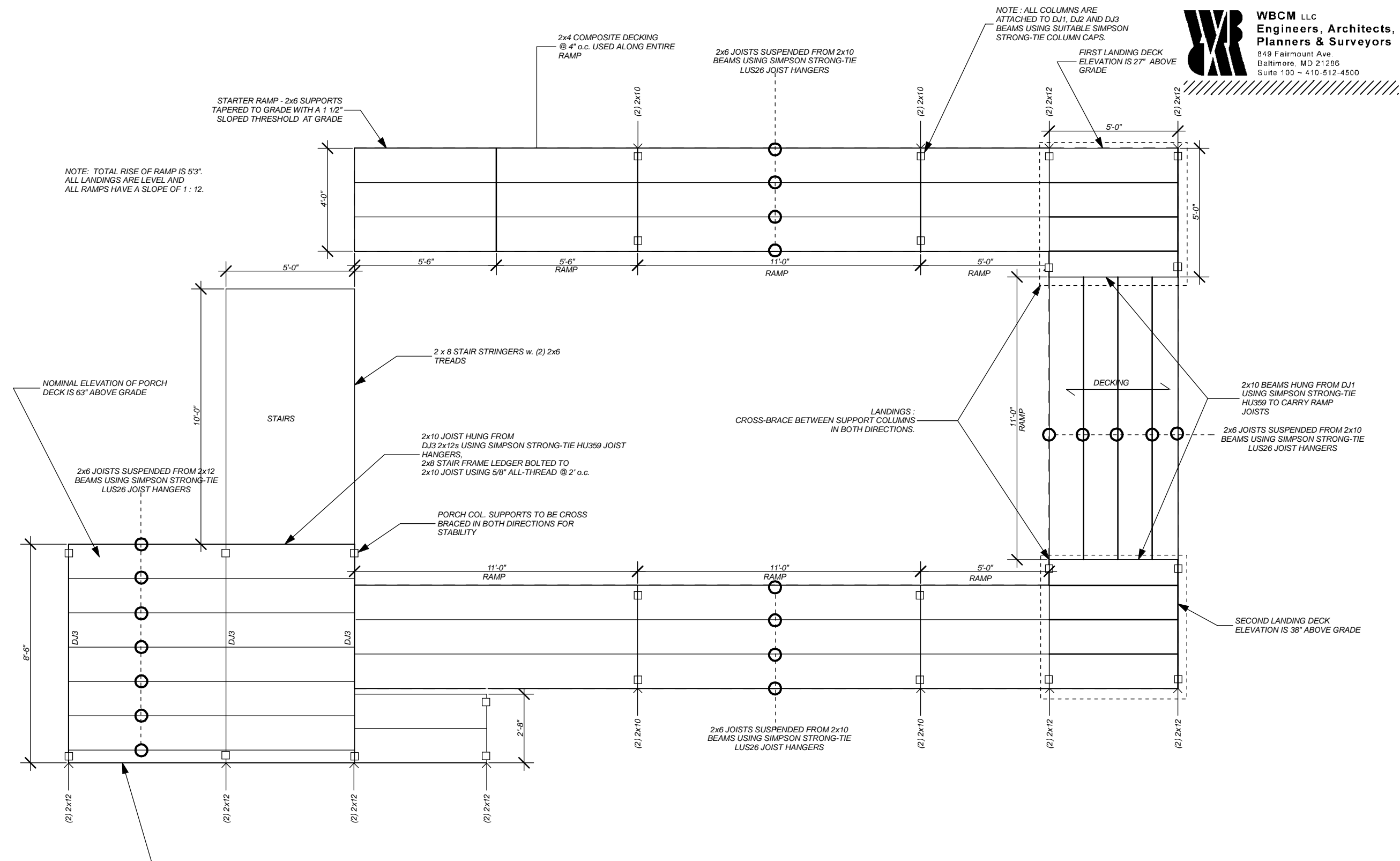
DATE: 08/12/05
CAD NAME: 08.06.05
DESIGNED BY: UMD
DRAWN BY: UMD
CHECKED BY: RWM
COPYRIGHT: UMD

SHEET TITLE

Porch / Ramp

S-13

SHEET 38 OF 57



1 S13 PORCH & RAMP FRAMING
 1/4" = 1'-0"

2x10 LEDGER HUNG FROM DOUBLE DJ3 2x12s USING SIMPSON STRONG-TIE HU359 JOIST HANGERS, AND BOLTED TO HOUSE BASE NORTH SHEAR WALL USING 5/8" ALL THREAD @ 2' o.c.

NOTE: TOTAL RISE OF RAMP IS 5'3". ALL LANDINGS ARE LEVEL AND ALL RAMPS HAVE A SLOPE OF 1 : 12.

NOTE: ALL COLUMNS ARE ATTACHED TO DJ1, DJ2 AND DJ3 BEAMS USING SUITABLE SIMPSON STRONG-TIE COLUMN CAPS.

2x10 BEAMS HUNG FROM DJ1 USING SIMPSON STRONG-TIE HU359 TO CARRY RAMP JOISTS

2x6 JOISTS SUSPENDED FROM 2x10 BEAMS USING SIMPSON STRONG-TIE LUS26 JOIST HANGERS

2x6 JOISTS SUSPENDED FROM 2x10 BEAMS USING SIMPSON STRONG-TIE LUS26 JOIST HANGERS

2x10 JOIST HUNG FROM DJ3 2x12s USING SIMPSON STRONG-TIE HU359 JOIST HANGERS; 2x8 STAIR FRAME LEDGER BOLTED TO 2x10 JOIST USING 5/8" ALL-THREAD @ 2' o.c.

PORCH COL. SUPPORTS TO BE CROSS BRACED IN BOTH DIRECTIONS FOR STABILITY

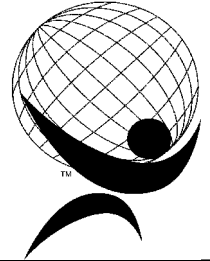
NOMINAL ELEVATION OF PORCH DECK IS 63" ABOVE GRADE

STARTER RAMP - 2x6 SUPPORTS TAPERED TO GRADE WITH A 1 1/2" SLOPED THRESHOLD AT GRADE

FIRST LANDING DECK ELEVATION IS 27" ABOVE GRADE

SECOND LANDING DECK ELEVATION IS 38" ABOVE GRADE

LANDINGS: CROSS-BRACE BETWEEN SUPPORT COLUMNS IN BOTH DIRECTIONS.



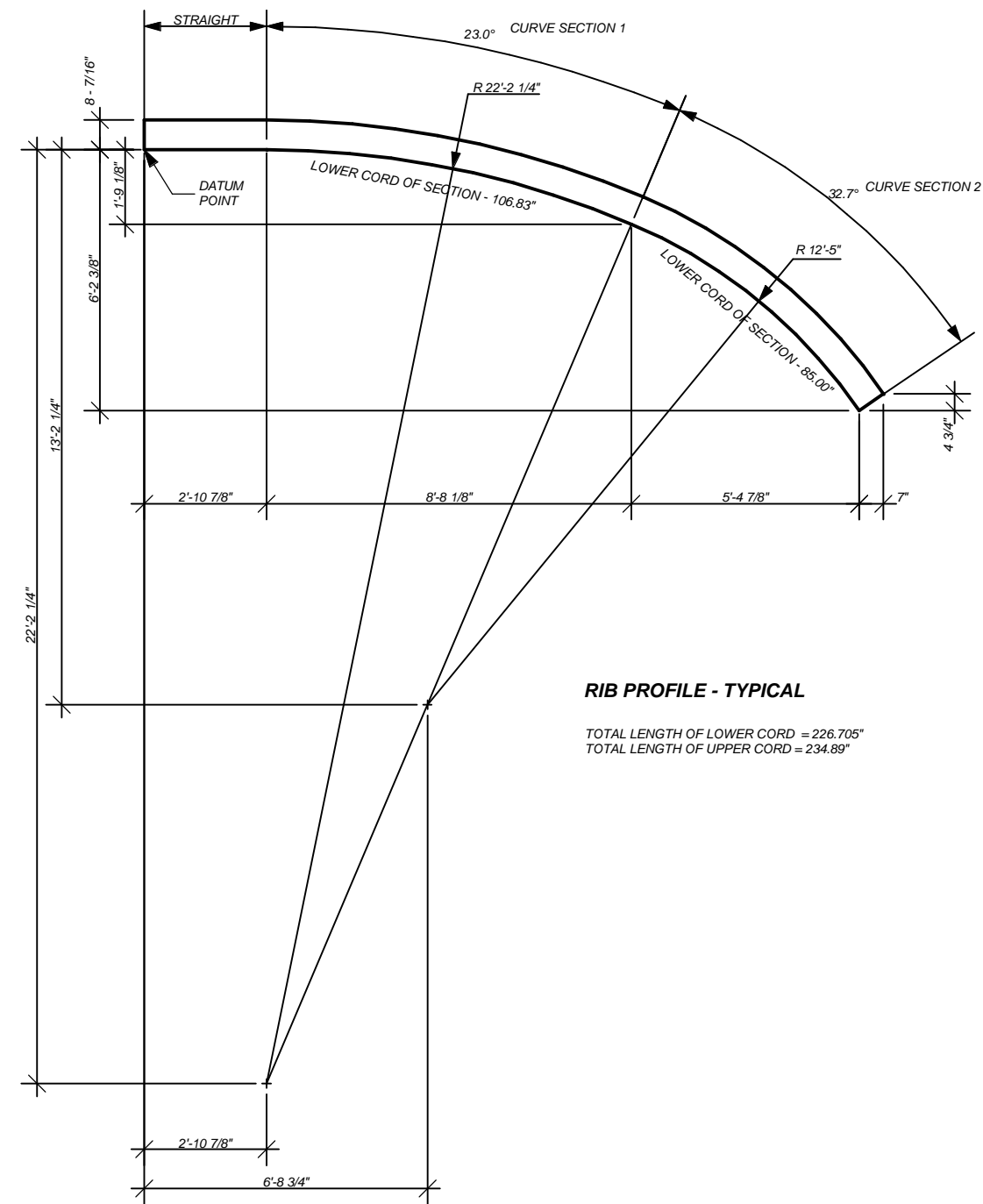
DATE: 08/12/05
 CAD NAME: 08.06.05
 DESIGNED BY: UMD
 DRAWN BY: UMD
 CHECKED BY: RWM
 COPYRIGHT: UMD

SHEET TITLE

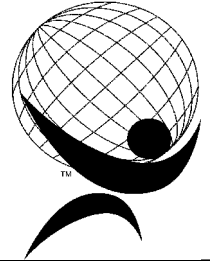
Rib - 1

S-14

SHEET 39 OF 57



1 S14 GLULAM RIB DESIGN
 1/4" = 1'-0"



DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY:RWM

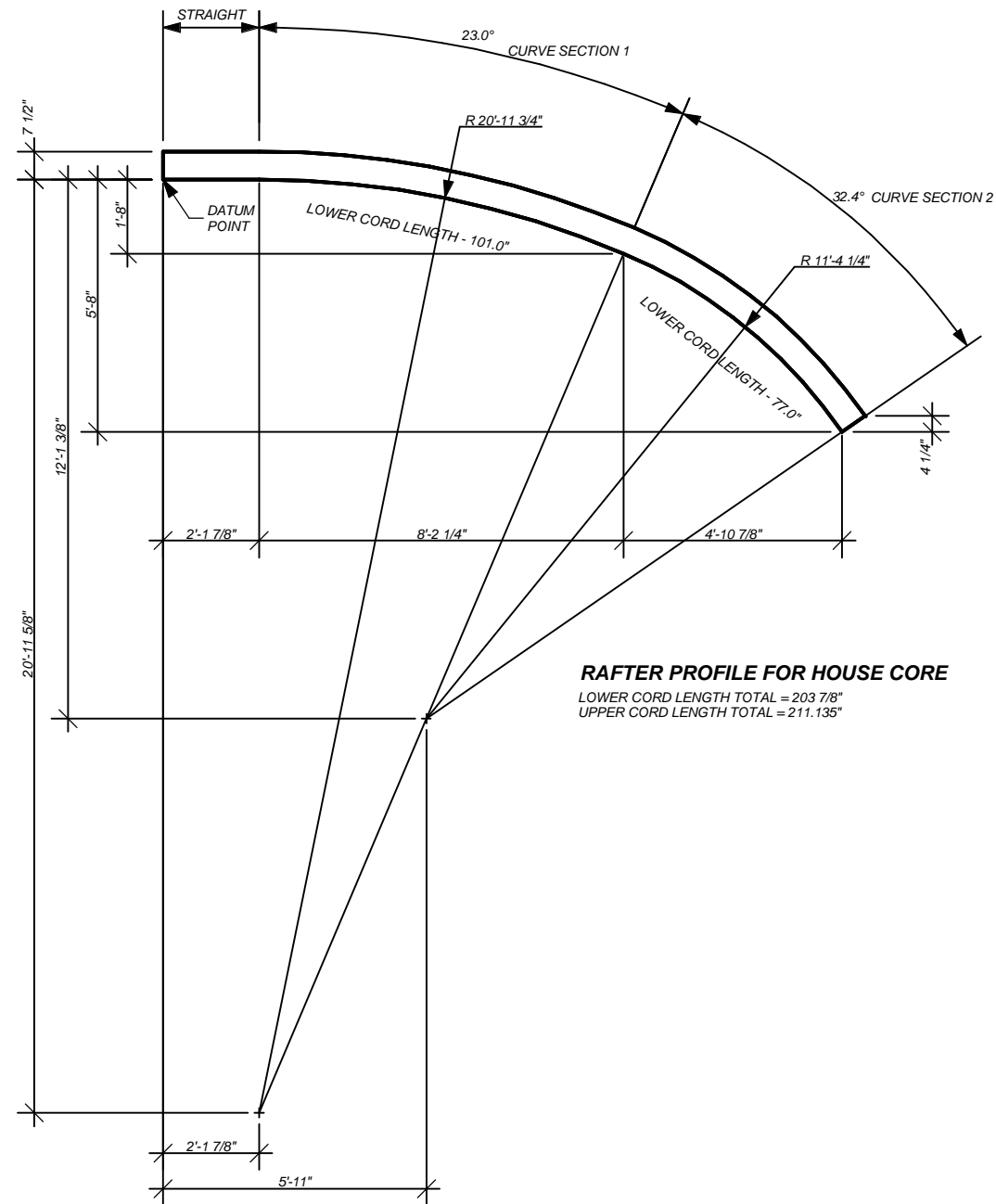
COPYRIGHT: UMD

SHEET TITLE

Rib - 2

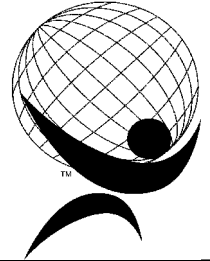
S-15

SHEET 40OF 57



RAFTER PROFILE FOR HOUSE CORE
 LOWER CORD LENGTH TOTAL = 203 7/8"
 UPPER CORD LENGTH TOTAL = 211.135"

1 S15 LONG GLULAM RAFTER DESIGN
 1/4" = 1'-0"



UNIVERSITY OF MARYLAND

University of Maryland
2005 Solar Decathlon



DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY: RWM

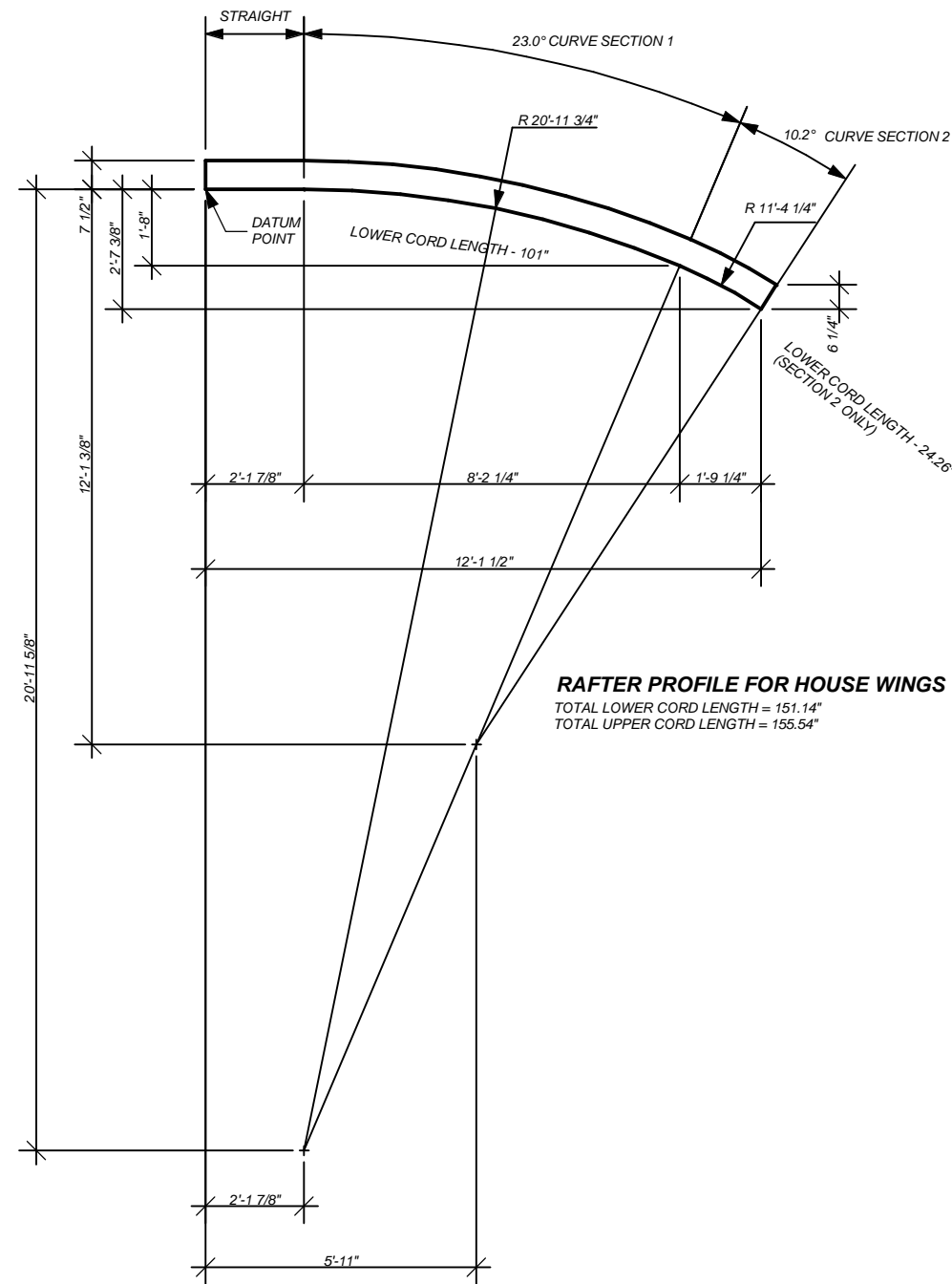
COPYRIGHT: UMD

SHEET TITLE

Rib - 3

S-16

SHEET 41 OF 57



1
S16 SHORT GLULAM RAFTER DESIGN
 1/4" = 1'-0"

GENERAL NOTES

1. CODE

- A. ALL CONSTRUCTION SHALL CONFORM TO THE PROVISIONS OF THE 2003 INTERNATIONAL BUILDING CODE, ITS SUPPLEMENTS AND THE MONTGOMERY COUNTY MARYLAND SUPPLEMENT.

2. DESIGN LOADING

- A. THE DESIGN DEAD LOAD FOR THE FRAMING IS AS FOLLOWS:

<u>FLOORS</u>		<u>ROOF</u>	
MEMBRANE ROOFING -----	1.0 PSF	FINISH -----	2.0 PSF
INSULATION -----	1.0 PSF	GYPSUM -----	30.0 PSF
STRUCTURAL FRAMING -----	3.0 PSF	SUBFLOOR -----	3.0 PSF
CEILING/FINISH -----	1.0 PSF	I-JOISTS -----	3.0 PSF
MISCELLANEOUS -----	<u>3.0 PSF</u>	INSULATION -----	3.0 PSF
		SHEATHING -----	<u>4.0 PSF</u>
TOTAL DEAD LOAD -----		TOTAL DEAD LOAD -----	
9.0 PSF		45.0 PSF	

B. FIRST FLOOR LIVE LOADS

FLOOR LIVE LOAD = 40 PSF
DECK LIVE LOAD = 100 PSF
PORCH LIVE LOAD = 40 PSF

C. ROOF LIVE LOADS

ROOF LIVE LOAD = 20 PSF

D. ROOF SNOW LOADS

GROUND SNOW LOAD (P_g) = 40 PSF
SNOW EXPOSURE FACTOR (C_e) = 1.3
SNOW LOAD IMPORTANCE FACTOR (I) = 1.0

SOLAR PANELS

ROOF SLOPE: $0^\circ < \Phi < 15^\circ$ - 40.04 PSF
 $15^\circ < \Phi < 40^\circ$ - 36.00 PSF
 $40^\circ < \Phi < 49^\circ$ - 26.00 PSF
 $49^\circ < \Phi < 70^\circ$ - 36.00 PSF
 $\Phi > 70^\circ$ - 0.00 PSF

NORTH LOW FLAT ROOF

AT HIGH EAVE - 50.0 PSF
AT LOW EAVE - 40.0 PSF

E. WIND LOAD

BASIC WIND SPEED (V) = 90 MPH
WIND LOAD IMPORTANCE FACTOR (I) = 1.0
WIND EXPOSURE = C
WIND DESIGN PRESSURE (P) = 15.86 PSF

F. EARTHQUAKE DESIGN DATA

$I_e = 1.25$
 $S_s = 0.199g$
 $S_1 = 0.101g$
SEISMIC HAZARD EXPOSURE GROUP = I
SEISMIC PERFORMANCE CATEGORY = B
SITE CLASS = D
SEISMIC FORCE RESISTING SYSTEM - WOOD SHEAR WALLS
DESIGN BASE SHEAR, V = 2.63 kips
RESPONSE MODIFICATION FACTOR (R) = 7
DEFLECTION AMPLIFICATION FACTOR (C_d) = 4.5
ANALYSIS PROCEDURE - SIMPLIFIED

- G. THE STABILITY OF THE STRUCTURE IS DEPENDENT UPON THE DIAPHRAGM ACTION OF THE FLOORS, ROOFS AND SHEAR WALLS ABOVE AND BELOW THE FLOOR FRAMING.

3. GENERAL

- A. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS BEFORE PROCEEDING WITH CONSTRUCTION. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.

- B. ALL WORK SPECIFIED HEREIN SHALL BE INSPECTED IN ACCORDANCE WITH THE BUILDING CODE AND ALL LOCAL ORDINANCES. THE ENGINEER MAY VISIT THE SITE TO ASCERTAIN GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS. SUCH SITE VISITS ARE NOT TO BE CONSTRUED AS MEETING ANY INSPECTION REQUIREMENTS UNLESS THE ENGINEER SPECIFICALLY SO STATES IN WRITING.

4. FOUNDATIONS

- 4A. TEMPORARY SPREAD FOOTING FOUNDATIONS AT U OF M CAMPUS AND NATIONAL MALL

- A. COEFFICIENTS OF FRICTION
SMOOTH CONCRETE ON BLACKTOP = 0.60
SMOOTH CONCRETE ON WET LUSH GRASS = 0.30

- B. ASSUMED DESIGN SOIL BEARING VALUE
WIND VELOCITY = 50 MPH, $F_{BRG.} = 1,000$ PSF
WIND VELOCITY = 90 MPH, $F_{BRG.} = 1,350$ PSF

- 4B. FINAL SPREAD FOOTING FOUNDATIONS AT RED WIGGLER FARM

- A. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-6" BELOW FINISH GRADE OR 1'-0" BELOW EXISTING GRADE, WHICHEVER IS GREATER, UNLESS NOTED OTHERWISE ON THE DRAWINGS. SEE SITE PLANS FOR EXISTING AND FINISHED GRADES.
B. TOP OF ALL INTERIOR FOOTINGS SHALL BE 8" BELOW SLAB-ON-GRADE OR EXISTING GRADE, WHICHEVER IS LOWER, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
C. ALL FOOTINGS HAVE BEEN DESIGNED FOR AN ASSUMED ALLOWABLE NET SOIL BEARING PRESSURE OF 1500 PSF. THE SOIL BEARING VALUE AT THE FOOTING DEPTH SHALL BE VERIFIED IN THE FIELD.
D. SLABS ON GRADE SHALL BE 4" THICK CONCRETE REINFORCED WITH 6 x 6, W1.4 x W1.4 OVER 6 MIL POLYETHYLENE AND 4" WASHED GRAVEL.

5. CAST-IN-PLACE CONCRETE

- 5A. GENERAL CONSTRUCTION

- A. ALL CONCRETE WORK SHALL CONFORM TO THE LATEST APPROVED (BY LOCAL GOVERNMENT) EDITIONS OF THE FOLLOWING A.C.I. DOCUMENTS:
a. ACI-301 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS
b. ACI-318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE
c. ASTM C94 READY-MIX CONCRETE
B. ALL CONCRETE, UNLESS NOTED OTHERWISE, SHALL BE STONE AGGREGATE CONCRETE HAVING THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTHS:
a. FOUNDATIONS, COLUMNS - 4000 PSI

- 5B. REINFORCING STEEL

- A. EXCEPT AS NOTED, ALL REINFORCING SHALL BE HIGH STRENGTH NEW BILLET STEEL CONFORMING TO ASTM DESIGNATION A-615 ($F_y=60,000$ PSI).
B. PROVIDE CONCRETE PROTECTION FOR REINFORCING BARS AS SHOWN ON THE SECTIONS AND DETAILS

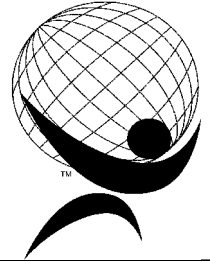
6. WOOD FRAMING

- 6A. GENERAL WOOD FRAMING

ALL WOOD FRAMING SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION SPECIFICATION AITC 105 "RECOMMENDED PRACTICE FOR THE ERECTION OF STRUCTURAL TIMBER FRAMING, AITC 106", "CODE OF STANDARD PRACTICE", AND "THE NATIONAL DESIGN



WBCM LLC
Engineers, Architects,
Planners & Surveyors
849 Fairmount Ave.
Baltimore, MD 21286
Suite 100 ~ 410-512-4500



UNIVERSITY OF
MARYLAND

University Of Maryland
2005 Solar Decathlon

DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY: RWM

COPYRIGHT: UMD

SHEET TITLE

Struct. Notes

S-17

SHEET 42 OF 57

- A. SPECIFICATION FOR WOOD CONSTRUCTION", LATEST EDITION, AS PUBLISHED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
- B. ALL DIMENSIONED LUMBER SHALL BE NO. 2 HEM FIR 19% MAXIMUM MOISTURE CONTENT OR BETTER.
- C. ALL STRUCTURAL GLUED LAMINATED TIMBER SHALL BEAR AN AITC QUALITY MARK OR BE ACCOMPANIED BY AN AITC CERTIFICATE OF CONFORMANCE CERTIFYING COMPLIANCE WITH THE SPECIFICATIONS.
- D. ALL GLUED LAMINATED MEMBERS SHALL BE FABRICATED FROM LAMINATIONS OF SOUTHERN PINE LUMBER TO THE DIMENSIONS AS INDICATED ON THE PLANS.
 - a. THE LAMINATING COMBINATION USED SHALL PROVIDE MINIMUM ALLOWABLE DESIGN UNIT STRESSES FOR DRY CONDITIONS OF USE (16" MAXIMUM MOISTURE CONTENT) AS FOLLOWS:
 - i. EXTREME FIBER IN BENDING $F_b = 2,200 \text{ PSI}$
 - ii. TENSION PARALLEL TO GRAIN $F_t = 1,000 \text{ PSI}$
 - iii. COMPRESSION PARALLEL TO GRAIN $F_{cl} = 1,500 \text{ PSI}$
 - iv. COMPRESSION PERPENDICULAR TO GRAIN $F_{cl} = 385 \text{ PSI}$
 - v. HORIZONTAL SHEAR $F_v = 200 \text{ PSI}$
 - vi. MODULUS OF ELASTICITY $E = 1,700,000 \text{ PSI}$
 - b. ADHESIVES FOR THE MAIN FLOOR GIRDERS AND CURVED ROOF JOISTS SHALL MEET THE REQUIREMENTS FOR DRY CONDITIONS OF USE (15% MAXIMUM MOISTURE CONTENT).
 - c. ADHESIVES FOR THE CURVED RIBS SHALL MEET THE REQUIREMENTS FOR WET CONDITIONS OF USE (15% MAXIMUM MOISTURE CONTENT).
- E. WOOD I-JOISTS SHALL BE AS DESIGNATED ON DRAWINGS AND AS MANUFACTURED BY WEYERHAUSER TRUSJOIST.
- F. LVL'S SHALL BE AS DESIGNATED ON DRAWINGS AND MANUFACTURED BY GEORGIA PACIFIC.
- G. ALL STEEL TIMBER FASTENINGS AND JOIST HANGERS SHALL BE A MINIMUM OF A 16 GA. GALVANIZED STEEL WITH A RATED LOAD CAPACITY EQUAL TO OR EXCEEDING THE IMPOSED LOADING REQUIREMENTS, UNLESS OTHERWISE SHOWN ON THE SECTIONS AND DETAILS.
- H. ALL WOOD PLATES BEARING ON MASONRY OR CONCRETE SHALL BE WOLMANIZED PRESSURE TREATED LUMBER AS MANUFACTURED BY KOPPERS COMPANY OR AN APPROVED ALTERNATE.
- I. ANCHOR ALL SILL PLATES TO MASONRY WALLS WITH 1/2"-DIA. X 1'-3" ANCHOR BOLTS SPACED AT 32' ON CENTER.

6B. WOOD RAFTERS, JOISTS, AND BEAMS

- A. PROVIDE ONE ROW OF BRIDGING BETWEEN ALL FLOOR JOISTS FOR EACH 8'-0" OF SPAN.
- B. PROVIDE TWO ROWS SOLID HORIZONTAL BRIDGING AT 4'-0" ON CENTER BETWEEN FIRST THREE EXTERIOR FLOOR JOISTS WHERE FRAMING RUNS PARALLEL TO BASEMENT MASONRY WALLS. ANCHOR BRIDGING TO SILL PLATE W/-TECO TIP-L-GRIPS.

6C. WOOD WALLS AND COLUMNS

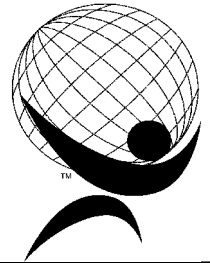
- A. ALL PERIMETER STUD BEARING WALLS AND INTERIOR BEARING WALLS SHALL BE 2X6 @ 16" MAX. O/C UNLESS NOTED OTHERWISE. PROVIDE SOLID BRIDGING AT MID HEIGHT OF ALL STUD WALLS TO 9'-0" HIGH UNLESS NOTED OTHERWISE.

PROVIDE DOUBLE STUDS AT ALL CORNERS, SIDES OF ALL OPENINGS, WINDOWS AND DOORS, AND BENEATH ALL WOOD BEAMS AND LINTELS UNLESS NOTED OTHERWISE

- A. ON PLANS.
- B. WHERE BLOCKING IS REQUIRED, BLOCKING SHALL BE THE SAME SIZE AS TYPICAL STUDS AND SHALL BE PROVIDED AT ALL JOINTS THAT ARE PERPENDICULAR TO STUDS UNLESS OTHERWISE SHOWN IN THE SECTIONS AND DETAILS.

6D. PLYWOOD AND WOOD DECKING

- A. ALL PLYWOOD SHALL MEET THE REQUIREMENTS OF THE PLYWOOD DESIGN SPECIFICATIONS AS PUBLISHED BY THE AMERICAN PLYWOOD ASSOCIATION.
- B. ENTIRE ROOF DECK SHALL BE 1/2"-32/16 PLYWOOD. PROVIDE 2X6 BLOCKING AT ALL EDGES PARALLEL TO SPAN. NAIL PLYWOOD ROOF DECK TO ROOF JOISTS AND 2X6 BLOCKING AS SHOWN ON THE SECTIONS AND DETAILS.
- C. ENTIRE SUB-FLOOR DECK SHALL BE 3/4"-24/32 PLYWOOD WOOD JOISTS. PROVIDE TONGUE AND GROOVES AT LONG EDGES OF PLYWOOD.
- D. PLYWOOD SHEATHING SHALL BE CONTINUOUS OVER A MINIMUM OF 2 SPANS.
- E. ALL PLYWOOD SHEATHING AND SUB-FLOORING SHALL BE GLUED TO THE STUDS AND JOISTS WITH A CONTINUOUS BEAD OF CONSTRUCTION ADHESIVE.
- F. UNLESS OTHERWISE SHOWN, PLYWOOD SHALL BE SECURELY FASTENED 3/8" FROM THE EDGE, NOT LESS THAN 6 INCHES ON CENTERS AT ALL EDGES, AND NOT LESS THAN 12 INCHES ON CENTER FOR ALL INTERMEDIATE JOISTS. USE 8D NAILS FOR 1/2" PLYWOOD AND 8D NAILS FOR 3/4" PLYWOOD.



UNIVERSITY OF MARYLAND

University of Maryland
2005 Solar Decathlon



DATE: 08/12/05

CAD NAME: 08.06.05

DESIGNED BY: UMD

DRAWN BY: UMD

CHECKED BY:RWM

COPYRIGHT: UMD

SHEET TITLE

Struct. Notes

S-18

SHEET 43OF 57